

## SDNV114 – Simulation of benchmark SAFE - T5 veil

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### Summary:

This test represents a simplified modeling of a study SAFE (Structure Armed Slightly Slim) under cyclic static loading. One is interested in the T5 veil. This test makes it possible to compare the answer of the structure modelled with the law `GLRC_DM` with the experimental results.

## 1 Problem of reference

### 1.1 Geometry

The studied geometry is that of the T5 structure of program SAFE [bib1]. The geometrical characteristics of the parts out of reinforced concrete are illustrated by [Figure 1.1-a]. They are made up of a veil and two wing walls (or partitions). The structure is also equipped with reported metal parts necessary to its setting under loading. These parts will not be modelled in this study.

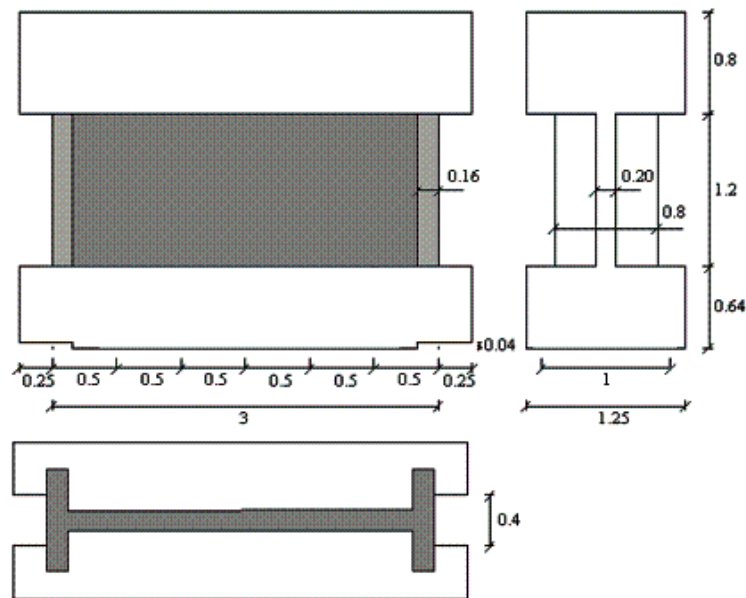
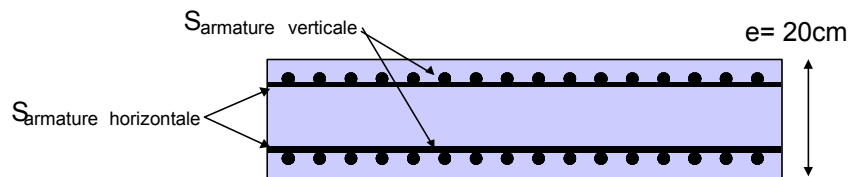


Figure 1.1-a: Geometry of the T5 model



Coupe dans le plan horizontal du mur central  
Figure 1.1-b: Illustration of reinforcement

The reinforcement of the model is composed of tablecloths of horizontal and vertical reinforcements placed on each of the two faces of the central wall, like in the returns [Figure 1.1-b]. According to the two horizontal and vertical directions, the rates of reinforcement  $r_h$  and  $r_v$  (quantity of reinforcement per linear meter of the veil) are identical and equal to 0,8% , that is to say:

$$\frac{S_{armatures\ horizontales}}{ml(\text{vertical})} = r_h e = \frac{0,8}{100} \cdot 20\text{cm} = 16\text{ cm}^2/ml$$

$$\frac{S_{armatures\ verticales}}{ml(\text{horizontale})} = r_v e = \frac{0,8}{100} \cdot 20\text{cm} = 16\text{ cm}^2/ml$$

And this for all two tablecloth faces North and South.

Maybe,  $8\text{ cm}^2/\text{ml}$  by tablecloth and direction (horizontal and vertical, that is to say  $2 \times 8 = 16\text{ cm}^2/\text{ml}$ ).

## 1.2 Properties of material

The behavior of the reinforced concrete is modelled via the law `GLRC_DM`. The parameters of law of behavior were readjusted to obtain an answer close to experimental measurements. The parameters materials of the law are given [Table 1.2-1] and [Table 1.2-2] for the principal veil and the two secondary veils (edges).

Effective Young modulus out of membrane	<code>E_M</code>	27 160 MPa
Effective Poisson's ratio out of membrane	<code>NU_M</code>	0,19
Effective Young modulus out of membrane	<code>E_F</code>	29 091 MPa
Effective Poisson's ratio out of membrane	<code>NU_F</code>	0,18
Thrust load of yield stress in traction	<code>NYT</code>	353 kN/m
Moment limits inflection	<code>MYF</code>	13 kN.m/m
Thrust load limits in compression	<code>NYC</code>	1 976 kN.m/m
Parameter of damage in traction	<code>GAMMA_T</code>	0,013
Parameter of damage in inflection	<code>GAMMA_F</code>	0,18
Parameter of damage in compression	<code>GAMMA_C</code>	0,9
Parameter of damage of coupling	<code>ALPHA_C</code>	1,1

**Table 1.2-1: Parameters of the model concrete for the principal veil**

Effective Young modulus out of membrane	<code>E_M</code>	27 574 MPa
Effective Poisson's ratio out of membrane	<code>NU_M</code>	0,19
Effective Young modulus out of membrane	<code>E_F</code>	29 598 MPa
Effective Poisson's ratio out of membrane	<code>NU_F</code>	0,18
Thrust load of yield stress in traction	<code>NYT</code>	286 kN/m
Moment limits inflection	<code>MYF</code>	8 kN.m/m
Thrust load limits in compression	<code>NYC</code>	1 627 kN.m/m
Parameter of damage in traction	<code>GAMMA_T</code>	0,015
Parameter of damage in inflection	<code>GAMMA_F</code>	0,209
Parameter of damage in compression	<code>GAMMA_C</code>	0,9
Parameter of damage of coupling	<code>ALPHA_C</code>	1,1

**Table 1.2-2: Parameters of the model concrete for the secondary veils**

## 1.3 Boundary conditions and loadings

Connection at the base:

The connection of the model with the low longitudinal beam was considered to be sufficiently stiff so that one models it by a perfect anchoring. Thus, all the nodes of the base of the model are blocked according to all the degrees of freedom.

#### Movements of the high longitudinal beam:

The purpose of the presence of the high longitudinal beam is to maintain the edge higher of the wall than horizontal by preventing rotations around the axis  $Y$ .

#### Loading:

The loadings taken into account are the actual weight of the structure as well as a displacement imposed at the top of the structure. The way of loading is given

## 1.4 Initial conditions

Nothing

## 2 Reference solution

### 2.1 Method of calculating

The reference solution corresponds to experimental measurements of forces according to imposed displacement.

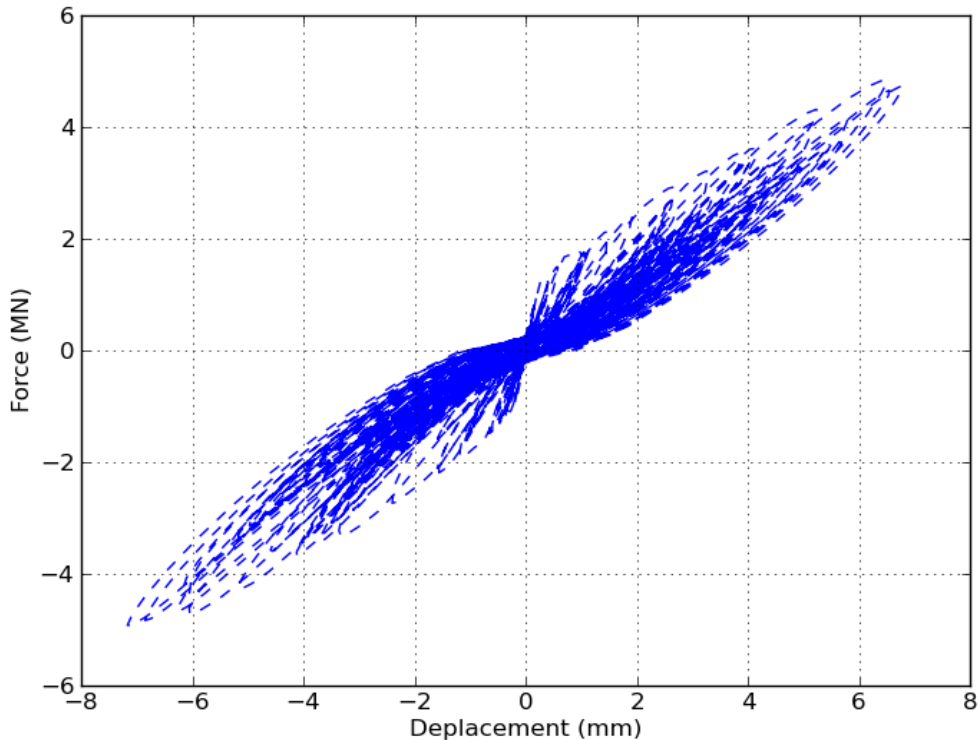


Figure 2.1-1: Answer of wall SAFE T5

### 2.2 Sizes and results of reference

The sizes tested are the horizontal force of reaction of the wall at various moments. The data are recapitulated in [Table 2.2-1].

Moment ( s )	Force ( MN )
11.07	2.9087
11.96	-3.2389
12.13	2.6812
13.09	-2,873
13.3	3.1698
13.47	-3.4344
14.03	2.3883
16.34	-3.1527
16.53	3.3595
16.72	-3.0954

Table 2.2-1: Sizes tested

## 2.3 Uncertainties on the solution

Digital solutions.

## 2.4 Bibliographical references

- [1] P. PEGON, G. MAGONETTE, F.J. MOLINA, G. VERZELETTI, T. DYNGLAND, P. NEGRO, D. TIRELLI, P. TOGNOLI, "Program SAFE: Report of the T5 test", Mechanical Unit of the Structures, Institute of the Systems, Data processing and Security, Joint Research Centre, European Commission, 21020 Ispra (Varese), Italy
- [2] S. GHAVANIAM, S. MILL, "Modeling of the T5 structure of program SAFE using Code\_Aster®", EDF R & D, H-T62-2006-04624-FR, 2006.

## 3 Modeling A

### 3.1 Characteristics of modeling

The various veils are modelled by elements `DKTG`. The link between the meshes of the central wall and the wing walls is made by the division of `Nœuds` on the level of the median layers.

### 3.2 Characteristics of the grid

The grid contains 308 elements of the type `QUAD4` and is represented on [Figure 3.2-1].

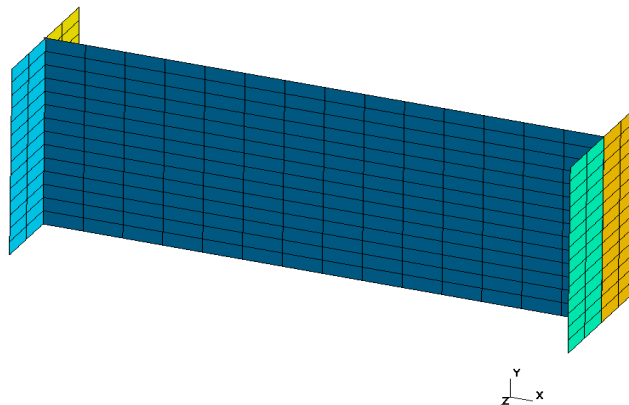


Figure 3.2-1: Grid of the T5 model

### 3.3 Sizes tested and results

One tests the horizontal force of reaction of the wall calculated on the higher edge of the veil.

Identification	Type of reference	Value of reference	Tolerance
Moment 11.07s	\SOURCE_EXTERNE \	2.9087E+06	2,00%
Moment 11.96s	\SOURCE_EXTERNE \	-3.2389E+06	6,00%
Moment 12.13s	\SOURCE_EXTERNE \	2.6812E+06	2,00%
Moment 13.09s	\SOURCE_EXTERNE \	-2.873E+06	10,50%
Moment 13.3s	\SOURCE_EXTERNE \	3.1698E+06	4,00%
Moment 13.47s	\SOURCE_EXTERNE \	-3.4344E+06	5,50%
Moment 14.03s	\SOURCE_EXTERNE \	2.3883E+06	10,00%
Moment 16.34s	\SOURCE_EXTERNE \	-3.1527E+06	9,00%
Moment 16.53s	\SOURCE_EXTERNE \	3.3595E+06	10,00%
Moment 16.72s	\SOURCE_EXTERNE \	-3.0954E+06	9,00%

### 3.4 Remarks

The modeling of wall SAFE thanks to the law of behavior `GLRC_DM` whose parameters were readjusted very satisfactory results in comparison with the experimental answer in term of effort and displacement give. However as one can see it on Figure 3.4-1, the loops of hysteresis are not

sufficiently important what makes it possible to propose not taken it in account of certain dissipative phenomena (friction of the lips of cracks of the concrete, steel-concrete connection,...).

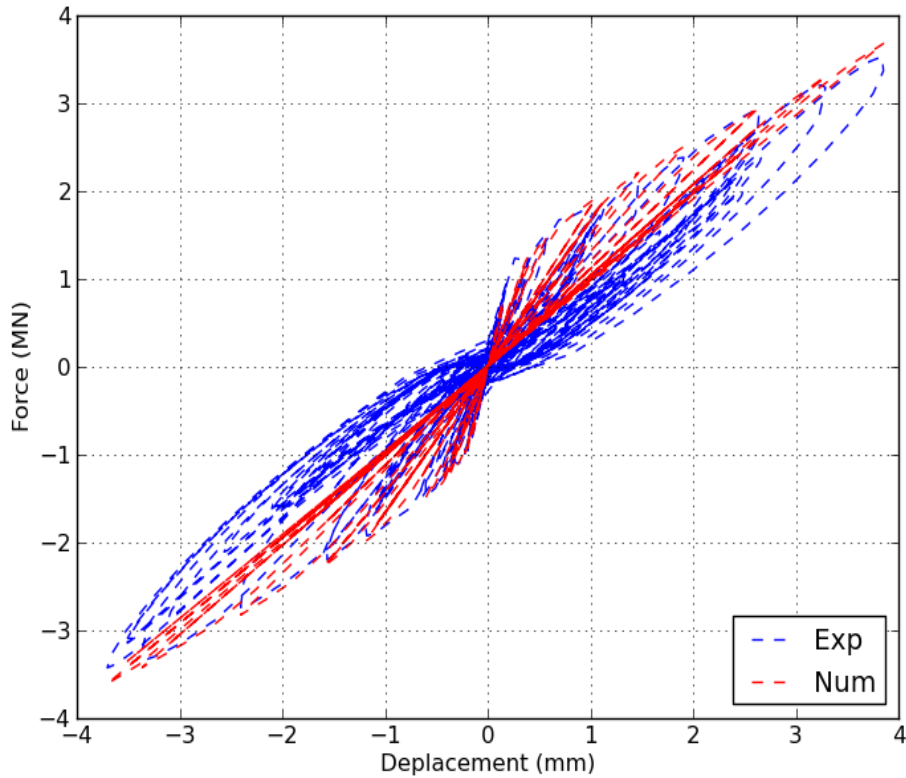


Figure 3.4-1: Comparison between the answer of model GLRC\_DM and the experimental data



## 4 Summary of the results

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The modeling installation makes it possible to find the experimental results satisfactorily.