

SDLL130 - Seismic response of a reinforced concrete beam (rectangular section) to linear behavior

Summary:

The problem consists in analyzing the seismic answer of a reinforced concrete beam via a modeling beam multifibre (POU_D_EM, modeling B).

Reference (modeling A) is calculated using *Code_Aster* with "classical" elements of beam Euler Bernoulli (POU_D_E).

1 General characteristics

1.1 Geometry

It is about a beam simply supported on its two supports [Figure 1.1-a].

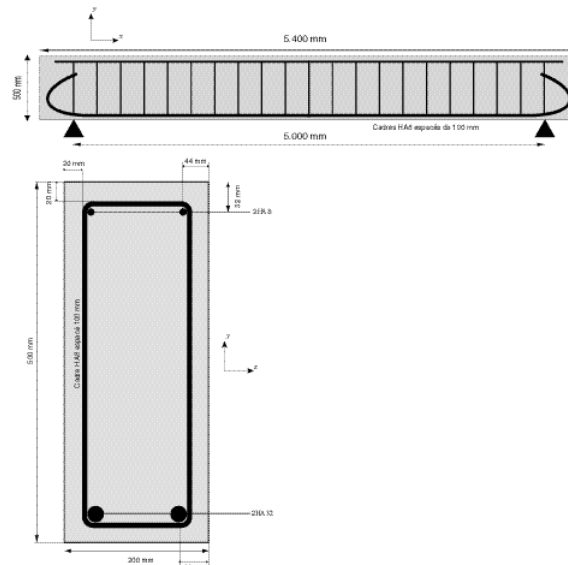


Figure 1.1-a: geometry of the structure

1.2 Material properties

- Concrete: $E = 37272 \text{ MPa}$, $\nu = 0.2$, $\rho = 2400 \text{ kg/m}^3$
- steel : $E = 200000 \text{ MPa}$, $\nu = 0.33$, $\rho = 7800 \text{ kg/m}^3$
- damping: of Rayleigh type ($\alpha K + \beta M$), with 5% on modes 1 and 2

1.3 Boundary conditions and loadings

Simple support in B : $dy = 0$

Support "doubles" in A : $dx = dy = 0$

To avoid the clean modes except plan, one blocks the following degrees of freedom on all the beam:
 $rx = ry = dz = 0$

Loading: earthquake $ac_s2_c_1$ [Figure 1.3-a], in the axis OY applied to the two supports (factor of amplification of the signal = 137).

NB: the transverse reinforcements are not taken into account in calculations

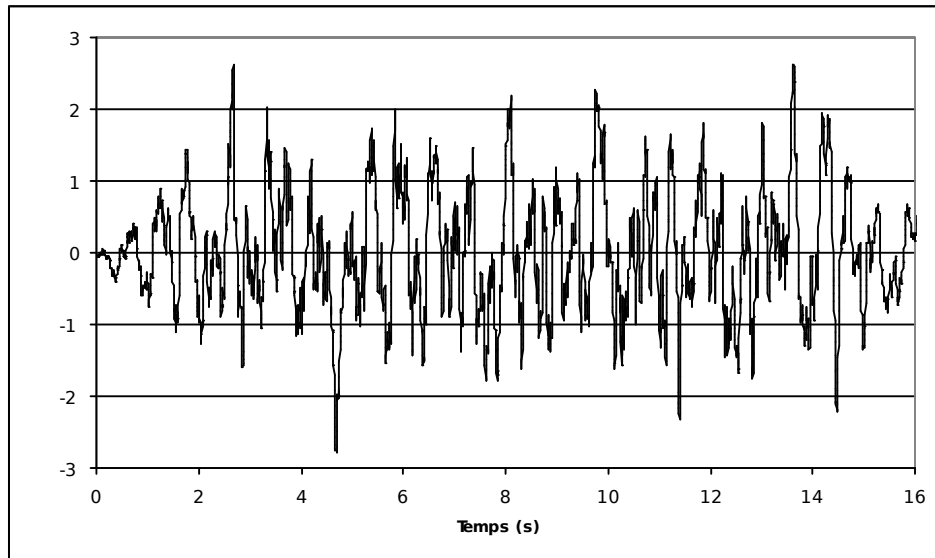


Figure 1.3-a: Accélérogramme ac_s2_c_1 imposed on the structure

2 Reference solution – Modeling A

The reference is obtained by a calculation *Code_Aster* with classical elements of beam of Euler (POU_D_E). The characteristics for this calculation of reference are obtained by homogenizing the steel-concrete section:

$$\text{Section: } S_{eq} = S_b + \frac{E_a}{E_b} S_a = 0,1 + \frac{200000}{37272} \times 0,0017 = 0,109 \text{ m}^2$$

$$\text{Quadratic moment: } I_{eq} = I_b + \frac{E_a}{E_b} I_a = 2,078 \cdot 10^{-3} + \frac{200000}{37272} \times 8,122 \cdot 10^{-5} = 2,514 \cdot 10^{-3} \text{ m}^4$$

The density selected is that of the concrete (the weight of steel is neglected).

3 Modeling B (POU_D_EM)

3.1 Characteristics of modeling

Longitudinal grid of the beam:

It is composed of 17 nodes and 16 pairs of elements POU_D_EM (16 elements for the concrete and 16 for steel).

Cross section of the beam:

The concrete is modelled by a grid (DEFI_GEOM_FIBRE/ SECT) composed of 2×20 quadrilaterals (40 fibres)

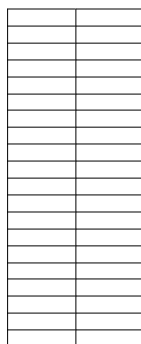


Figure 3.1-a: Discretization of the section

Steel is modelled by 4 specific fibres (DEFI_GEOM_FIBRE/FIBRE)

Coefficients α and β for damping are calculated using the following formula

$$\begin{pmatrix} \alpha \\ \beta \end{pmatrix} = 2 \frac{\omega_1 \omega_2}{\omega_2^2 - \omega_1^2} \begin{pmatrix} 1 & 1 \\ \omega_2 & \omega_1 \\ \omega_2 & -\omega_1 \end{pmatrix} \begin{pmatrix} \xi_1 \\ \xi_2 \end{pmatrix}$$

where ω_1 and ω_2 are the first two own pulsations ($\omega = 2\pi f$) and ξ_1 and ξ_2 are the depreciation wished on the first two modes.

With $f_1 = 37.8 \text{ Hz}$ and $f_2 = 149.2 \text{ Hz}$ (see paragraph [§4]), for modal depreciation of 5% , we find: $\alpha = 8.5 \cdot 10^{-5}$ and $\beta = 18.985$.

For the calculation of the temporal answer, the step of selected time is $1/100^{\text{ème}}$ from second.

3.2 Sizes tested and results

The curves of reaction according to time and marks with arrows in the center according to time are presented on the figures [Figure 4-a] with [Figure 4-d].

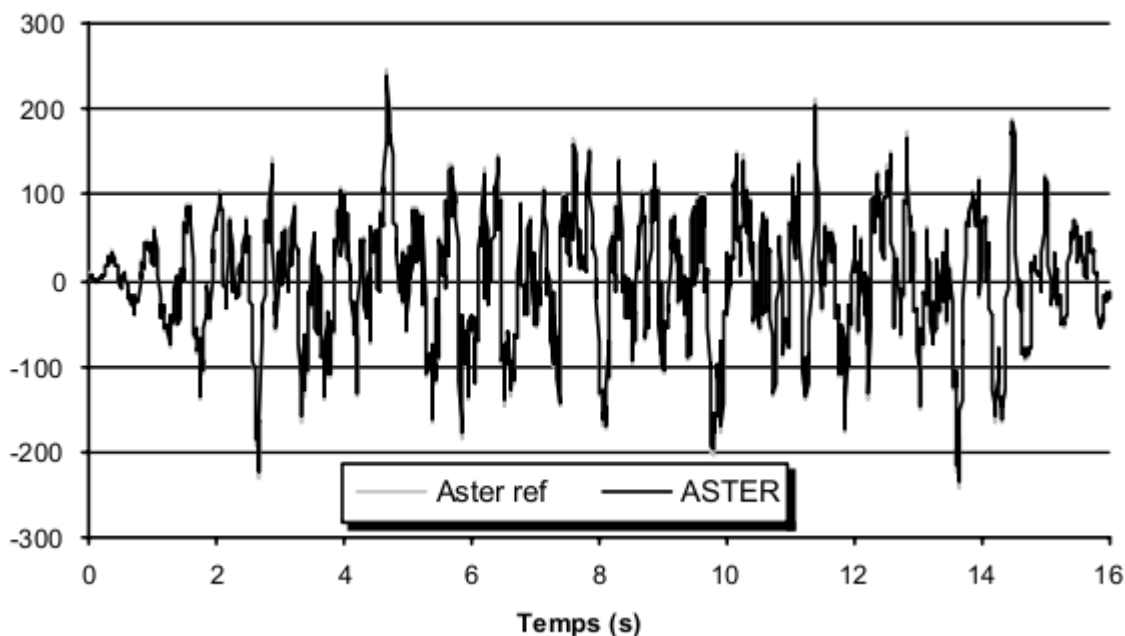


Figure 4-a: Reaction to the first supports according to time

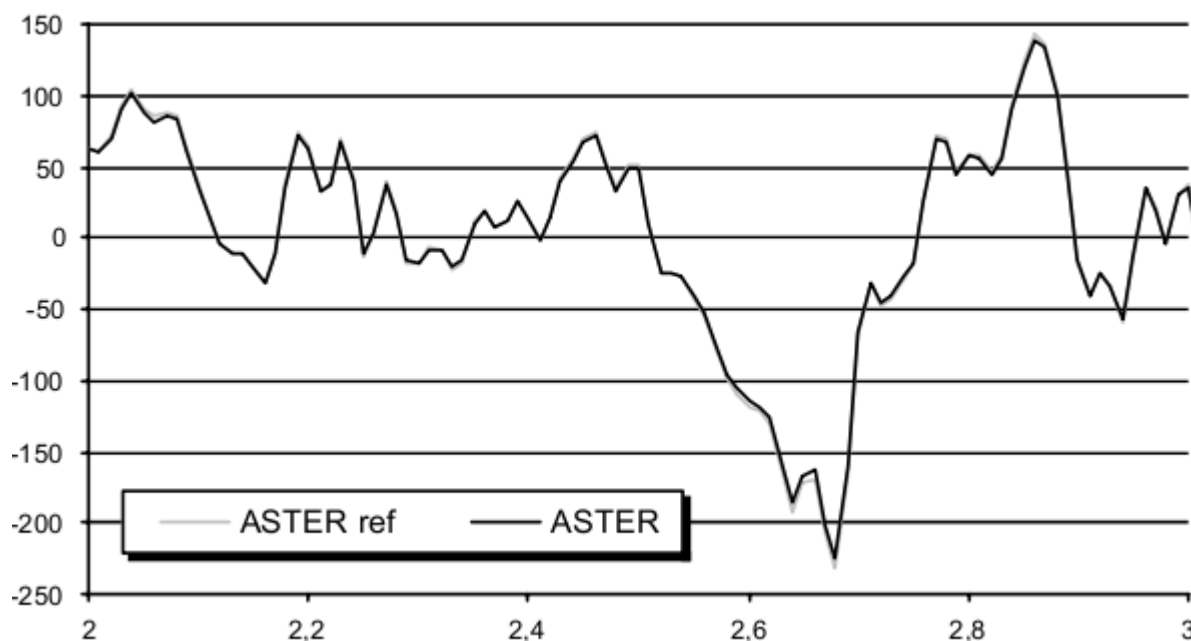


Figure 4-b: Detail of the reaction between 2 and 3 seconds

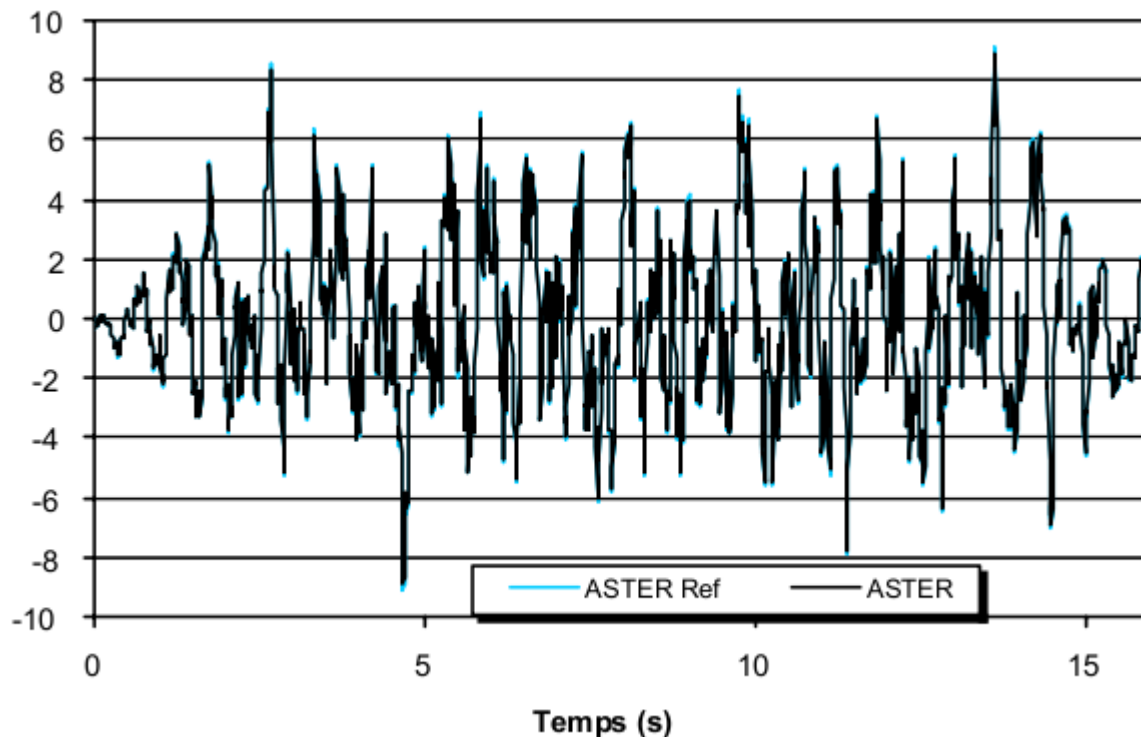


Figure 4-c: Arrow in the center according to time

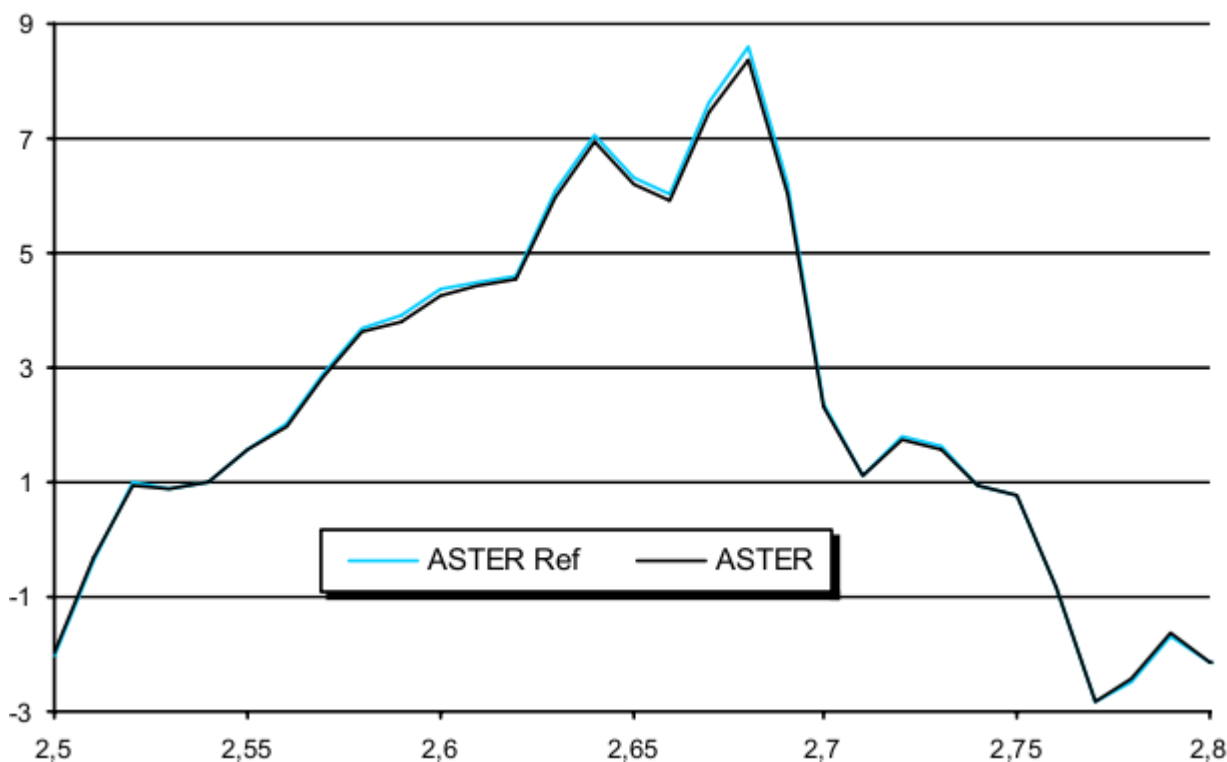


Figure 4-d: Detail of the arrow between 2.5 and 2.8 seconds

Tests of results (TEST_RESU) are carried out for the first three Eigen frequencies. One also tests the reaction on the first support and the arrow in the center is tested at the moments 1s (not 100) and

Warning : The translation process used on this website is a "Machine Translation". It may be imprecise and inaccurate in whole or in part and is provided as a convenience.

Copyright 2017 EDF R&D - Licensed under the terms of the GNU FDL (<http://www.gnu.org/copyleft/fdl.html>)

2s (not 200), then for the 2 first extremums of the curves, at the moments 2,68 s (not 268) and 4,68 s (not 468).

Eigen frequency	ASTER ref.	ASTER	Relative error %
1	37.80	37.83	0.07
2	149.20	149.28	0.05
3	200.30	200.39	0.04

REACTION	ASTER ref.	ASTER	Error relative %
1,00 s	1,8878.10 ⁴	1,8479.10 ⁴	2.1
2,00 s	6,3393.10 ⁴	6,2184.10 ⁴	1.9
2,68 s	- 2,3222.10 ⁵	- 2,2443.10 ⁵	3.4
4,68 s	2,4692.10 ⁵	2,3979.10 ⁵	2.9

MARKS WITH ARROWS	ASTER ref.	ASTER	Relative error %
1.00 S	-6,0694.10 ⁻⁴	-5,9846.10 ⁻⁴	1.4
2.00 S	-2,3507.10 ⁻³	-2,3362.10 ⁻³	0.6
2.68 S	8,5790.10 ⁻³	8,3929.10 ⁻³	2.2
4.68 S	-9,1084.10 ⁻³	-8,9530.10 ⁻³	1.7

4 Summary of the results

Results got using modeling beam multifibre (POU_D_EM) are in concord with the classical modeling of right beam of Euler (POU_D_E) of Code_Aster.