

SDNS106 – Transient response of a reinforced concrete slab: models GLRC_DAMAGE and GLRC_DM

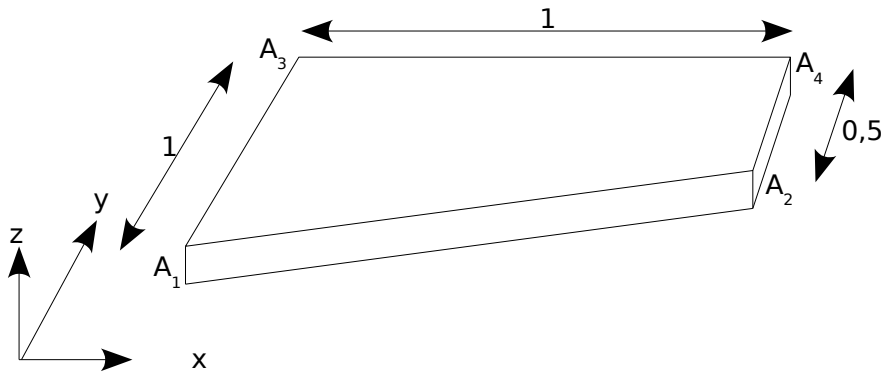
Summarized:

This test validates total models GLRC_DAMAGE [R7.01.31] and GLRC_DM [R7,01.32] applied to the explicit dynamics (DYNA_NON_LINE (SCHEMA_TEMPS=DIFF_CENT)). It is about a reinforced concrete plate trapezoidal, leaned on two opposite sides and requested in bending by a pressure.

1 Problem of reference

1.1 Geometry

the geometry used in this case test is a concrete plate reinforced with thickness $e=0.1\text{ m}$ and trapezoidal form.



Appear 1.1-a: Studied geometry

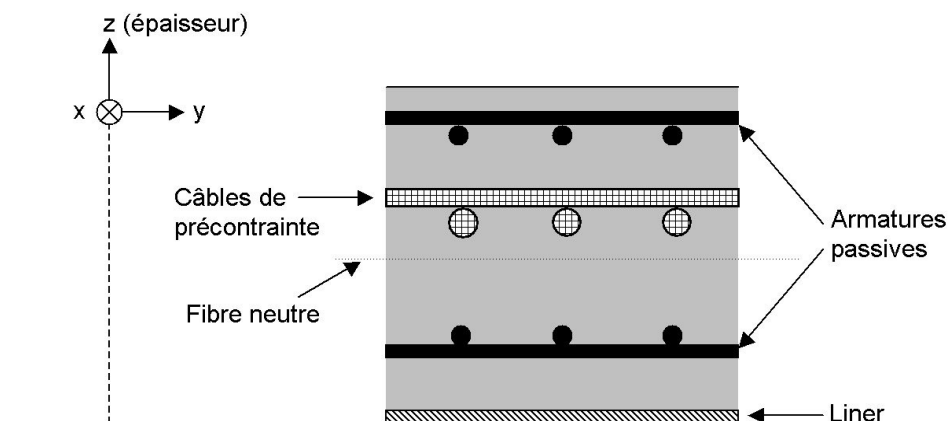
the characteristics of the concrete section reinforced are:

Higher three-dimensions function: section per following linear meter x and y $=5.65 \cdot 10^{-4} \text{ m}^2 / \text{ml}$; eccentring compared to the following average average x and y : $+0.0475 \text{ m}$ (either 95% of the thickness),

lower Three-dimensions function: section per following linear meter x and y $=5.65 \cdot 10^{-4} \text{ m}^2 / \text{ml}$; eccentring compared to the following average average x and y : -0.0475 m (either 95% of the thickness),

Cables of prestressed: section per following linear meter $x=4.56 \cdot 10^{-3} \text{ m}^2 / \text{ml}$ and $y=1.32 \cdot 10^{-2} \text{ m}^2 / \text{ml}$; no the eccentring compared to the average average; prestressed according to x and $y = -3 \text{ MN}$,

Liner: the thickness of the liner is of 6 mm and is positioned on the lower face.



Appear 1.1-b: Section of the reinforced concrete Properties plate

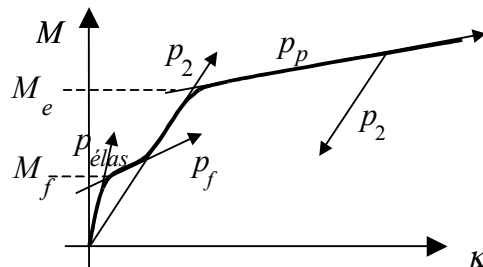
1.2 of the materials

the characteristics of the various materials for modelization `GLRC_DAMAGE` are summarized in the table which follows.

Material	Modulus Young MPa	Poisson's ratio	Density kg/m ³	Slope hardening	of Yield stress in tension MPa	Yield stress in compression MPa
Concrete	30000.	0.2	2500	0	5.-35	
Steel of reinforcements	200000	-	-	0	3000	-3000
Steel of the liner and the cables of prestressing	200000	-	-	0.500		-500

to supplement constitutive law `GLRC_DAMAGE`, it is necessary to define the globalized parameters of homogenized model.

Parameters	Values
Γ	0
Q_{P1}	0.15
Q_{P2}	0.15
C_N	87.3 MPa
C_M	14.8 MPa



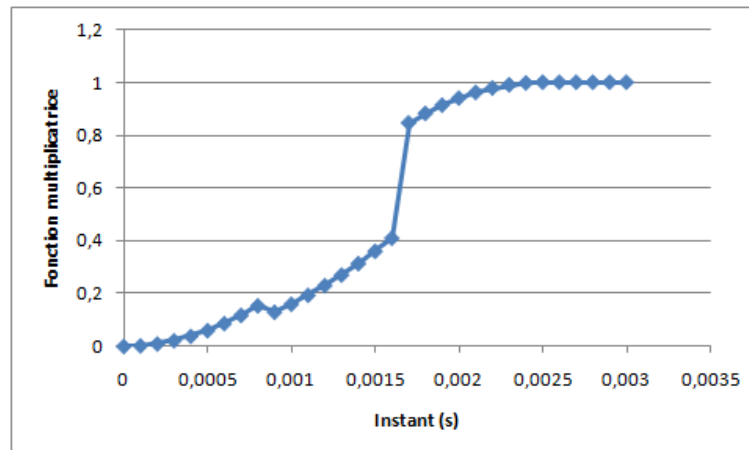
Appears 1.2-a: Curve moment – curvature of the behavior of a reinforced concrete plate in bending.

The characteristic materials for modelization `GLRC_DM` are summarized in the table which follows.

Parameters	Values
E_{eq}^m	30000 MPa
ν_m	0.22
E_{eq}^f	73000 MPa
ν_f	0.24
γ_{mt}	0.02
γ_f	0.05
N_D	470000 N/m
M_D	16000 N

1.3 Boundary conditions and loadings

On the corner AI of the plate, one embeds displacements $u_x = u_y = u_z = 0$, as well as rotations $\theta_x = \theta_y = \theta_z = 0$. Displacements are blocked according to x and z on the sides $AIA3$ and $A2A4$. A pressure is applied to the group of slab in the direction $(0,0,0,1,0)$ and applies $F_0 = 20 \cdot 10^7 N$ to modelization A. For the modelizations B and C, one applies a nodal force to the group of slab $1500 N$. This force is applied in a progressive way while following the multiplying function represented to the figure which follows.



Appear 1.3-a: Multiplying function of the loading for the modelizations B and C

1.4 Initial conditions

IN an initial state, displacements and the velocities are worth zero everywhere.

2 Reference solution

2.1 Méthode de calcul

the values of reference are obtained by comparisons with EUROPLEXUS for modelization A. For the modelizations B and C, the values of reference are the non regression ones.

2.2 Quantities and results of reference

the results of reference for the modelization A are recapitulated in the table which follows. The data are obtained at the point P_{04} (cf appears 3.1-a).

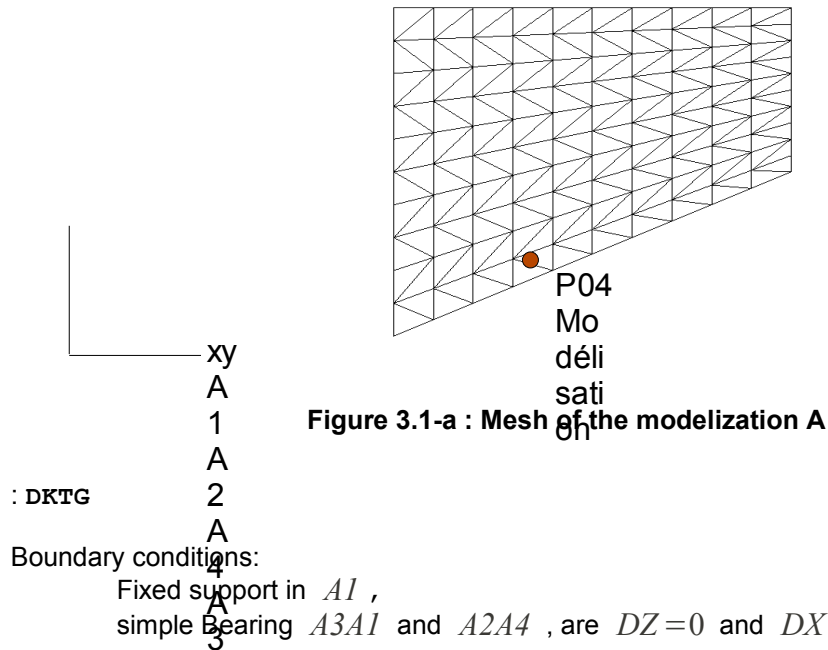
Quantities	Reference
Displacement following z to $t=2 \cdot 10^{-5} s$	$-1,74913 \cdot 10^{-4} m$
Acceleration following z to $t=2 \cdot 10^{-5} s$	$-7,99968 \cdot 10^5 m.s^{-2}$
following Displacement z at $t=1 \cdot 10^{-3} s$	$-4,4933 \cdot 10^{-1} m$
Velocity following z to $t=1 \cdot 10^{-3} s$	$-8,24761 \cdot 10^2 m.s^{-1}$
Elastic strain energy to the node 2 of the mesh 1 to $t=2 \cdot 10^{-6} s$	$1,46067 \cdot 10^{-1} J$

2.3 Uncertainties on the solution

Comparisons with EUROPLEXUS for the temporal responses in displacement, the reactions, and kinetic energy, for a sinusoidal loading

3 Modelization A

3.1 Characteristic of the modelization



Boundary conditions:
Fixed support in $A1$,
simple Bearing $A3A1$ and $A2A4$, are $DZ=0$ and $DX=0$.

Temporal integration:
Diagram: DIFF_CENT, formulation: ACCELERATION,
Time step: $2 \cdot 10^{-6} s$.

3.2 Characteristics of the mesh

Many nodes: 121, Number of meshes: elements TRI3 : 200, elements SEG2 : 40.

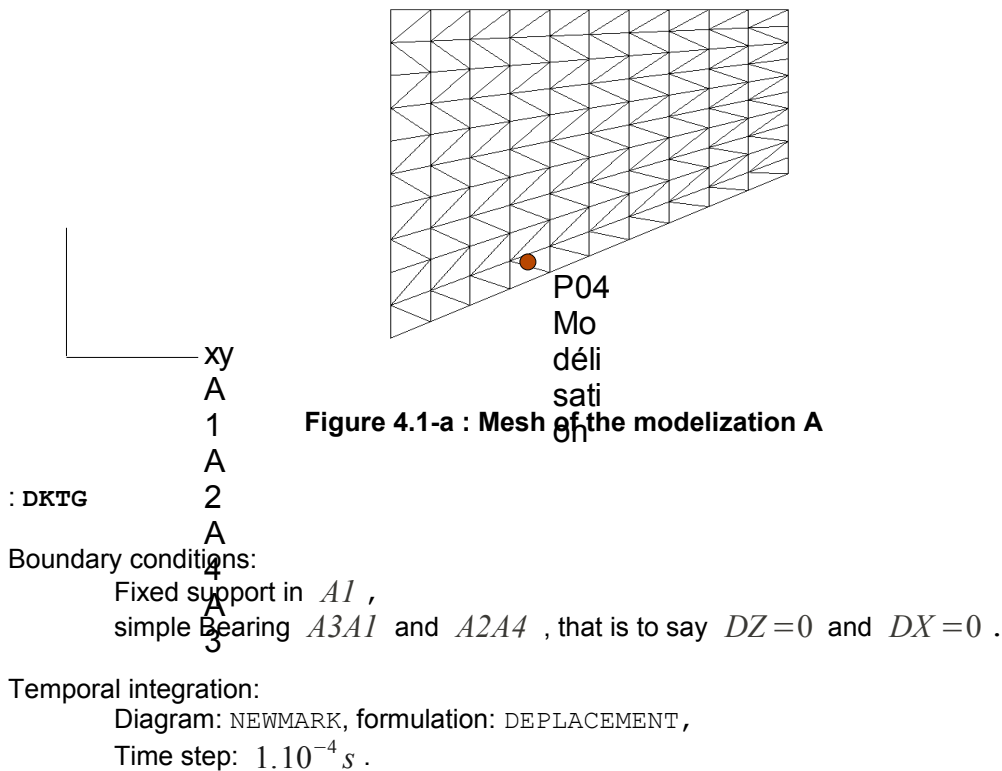
3.3 Quantities tested and results

One tests displacements, velocity and acceleration of the point $P04$ at various times. One tests also elastic strain energy in two points of structure.

Standard	identification of reference	Value of reference	Tolerance
Displacement following z with $t=2 \cdot 10^{-5} s$	"AUTRE_ASTER"	$-1,74913 \cdot 10^{-4} m$	0,15
Acceleration following z with $t=2 \cdot 10^{-5} s$	"AUTRE_ASTER"	$-7,99968 \cdot 10^5 m.s^{-2}$	10^{-4}
following Displacement z with $t=1 \cdot 10^{-3} s$	"AUTRE_ASTER"	$-4,4933 \cdot 10^{-1} m$	0,03
Velocity following z with $t=1 \cdot 10^{-3} s$	"AUTRE_ASTER"	$-8,24761 \cdot 10^2 m.s^{-1}$	0,1
formulates Elastic strain energy with the node 2 1 with $t=2 \cdot 10^{-6} s$	"AUTRE_ASTER"	$1,46067 \cdot 10^{-1} J$	10^{-5}

4 Modelization B

4.1 Characteristic of the modelization



4.2 Characteristics of the mesh

Many nodes: 121, Number of meshes: elements QUAD4 : 100, elements SEG2 : 40.

4.3 Quantities tested and results

One tests displacements, velocity and acceleration of the point $P04$ at various times.

Standard	identification of reference	Value of reference	Tolerance
Displacement following z at $t=2.5 \cdot 10^{-3} s$	Non regression	$-4.09238 \cdot 10^{-4} m$	10^{-5}
Velocity according to z to $t=2.5 \cdot 10^{-3} s$	formula	$-0.4984907 m.s^{-1}$	10^{-5}
Acceleration according to z with $t=2.5 \cdot 10^{-3} s$	formula	$-56,1819 m.s^{-2}$	10^{-5}
following Displacement z with $t=5 \cdot 10^{-3} s$	formula	$-1.89876 \cdot 10^{-4} m$	10^{-5}
Velocity according to z with $t=5 \cdot 10^{-3} s$	formula	$0.3652467 m.s^{-1}$	10^{-5}
formulates Acceleration z formula $t=5 \cdot 10^{-3} s$	Non regression	$-797.416 m.s^{-2}$	10^{-5}

5 formula Modelization

5.1 Characteristic of the modelization

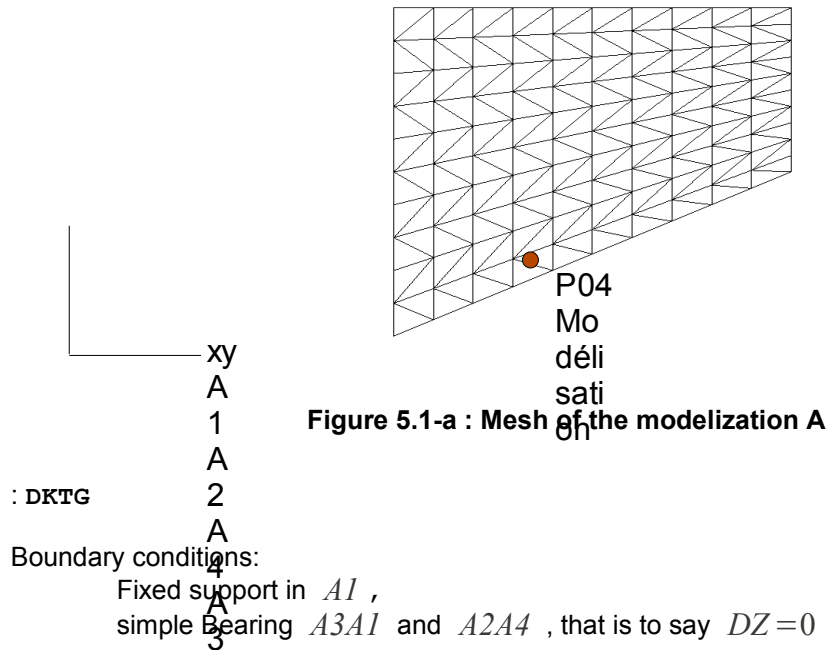


Figure 5.1-a : Mesh of the modelization A

: DKTG

Boundary conditions:

Fixed support in $A1$,
simple Bearing $A3A1$ and $A2A4$, that is to say $DZ=0$ and $DX=0$.

Temporal integration:

Diagram: NEWMARK, formulation: DEPLACEMENT,
Time step: $1.10^{-4}s$.

5.2 Characteristics of the mesh

Many nodes: 121, Number of meshes: elements TRI3 : 200, elements SEG2 : 40.

5.3 Quantities tested and results

One tests displacements, velocity and acceleration of the point $P04$ at various times. One tests also elastic strain energy in two points of structure.

Standard	identification of reference	Value of reference	Tolerance
following Displacement z with $t=2.5 \cdot 10^{-3} s$	formula	$-3.71031 \cdot 10^{-4} m$	10^{-5}
Velocity according to z with $t=2.5 \cdot 10^{-3} s$	formula	$-0.4496707 m.s^{-1}$	10^{-5}
Acceleration according to z with $t=2.5 \cdot 10^{-3} s$	formula	$-61,3004 m.s^{-2}$	10^{-5}
following Displacement z with $t=5 \cdot 10^{-3} s$	formula	$-1.56827 \cdot 10^{-4} m$	10^{-5}
Velocity according to z with $t=5 \cdot 10^{-3} s$	formula	$-0.4552156 m.s^{-1}$	10^{-5}
Acceleration according to z with $t=5 \cdot 10^{-3} s$	formula	$73.0819 m.s^{-2}$	10^{-5}

6 Summary of the results

the results got with *Code_Aster* are close to those obtained with EUROPLEXUS. This modelization thus validates the use of `GLRC_DAMAGE` with *Code_Aster*.