

## SSLL104 - Predeformations in a straight beam

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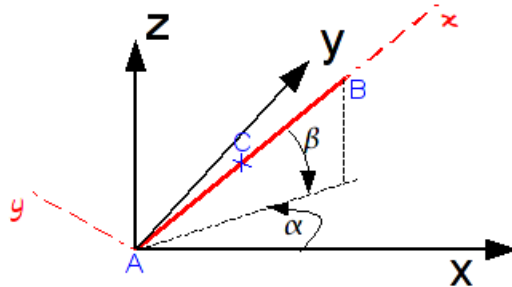
### Summarized:

This test validates the taking into account of predeformations in the elastic design of a straight beam. The characteristics of computation are:

- static analysis,
- linear behavior,
- model linear,
- 1 only modelization testing elements `POU_D_E`, `POU_D_T`, `POU_D_TG` and `POU_D_EM`
- the solution is analytical.

## 1 Problem of reference

### 1.1 Geometry



a beam AB length  $l = 100 \text{ mm}$  is located on trisecting trihedron  $(X, Y, Z)$  : the coordinates of the point B are:  $B = \left( \frac{100}{\sqrt{3}}, \frac{100}{\sqrt{3}}, \frac{100}{\sqrt{3}} \right)$

One C defines also a point medium of A, B.

the local coordinate system  $(A, x, y, z)$  results from the total reference  $(A, X, Y, Z)$  by the

$$\text{nautical angles} \begin{cases} \alpha = 45^\circ \\ \beta = -35.26^\circ \text{ solution de } \cos \beta = \sqrt{\frac{2}{3}} \end{cases}$$

### 1.2 Material properties

the material is elastic linear.

Young modulus  $E = 1.0 \text{ MPa}$  (without influence on result).

Poisson's ratio:  $\nu = 0$

### 1.3 Boundary conditions and loadings

Fixed support in  $A$  :  $DX = DY = DZ = DRX = DRY = DRZ = 0$  .

Loading: predeformation in the local coordinate system  $(A, x, y, z)$

- elongation according to  $x$  :  $\epsilon_x^0 = 0.001$
- curvature around  $y$  :  $\chi_y^0 = 0.002$
- curvature around  $z$  :  $\chi_z^0 = 0.003$

### 1.4 Characteristics of the section of beam

All the characteristics (area, inertias,...) are taken equal to 1. They are without influence on result.

## 2 Reference solution

### 2.1 Method of calculating used for the reference solution

the solution is analytical. It is calculated in the local coordinate system.  
that is to say:

$U = (u, v, w, \theta_x, \theta_y, \theta_z)$  the displacement of the beam and  $E = (\epsilon_x, \chi_y, \chi_z, \gamma_{xy}, \gamma_{xz})$  generalized strain.

That is to say the solution:

$$u = \alpha x \quad v = \gamma \frac{x^2}{2} \quad w = -\beta \frac{x^2}{2} \quad \theta_x = 0 \quad \theta_y = \beta x \quad \theta_z = \gamma x$$

then:

$$\epsilon_x = u_{,x} = \alpha \quad \chi_y = \theta_{y,x} = \beta \quad \chi_z = \theta_{z,x} = \gamma \quad \gamma_{xy} = v_{,x} - \theta_z = 0 \quad \gamma_{xz} = w_{,x} + \theta_y = 0$$

If one chooses  $\alpha = \epsilon_x (= 0.001)$   $\beta = \chi_y^0 (= 0.002)$ ,  $\gamma = \chi_z^0 (= 0.003)$  then  $E - E_{init} = 0$  and the forces are null: the equilibrium is thus checked. In addition, the solution checks the boundary conditions of fixed support in  $A$ . It is thus the solution of the posed problem.

### 2.2 Results of reference

the results expressed in the local coordinate system are:

In  $B$  :

$$Dx = 0.10 \text{ mm} ; Dy = 15.0 \text{ mm} ; Dz = -10.0 \text{ mm} ; DRx = 0.0 \text{ rd} ; DRY = 0.2 \text{ rd} ; DRz = 0.30 \text{ rd}$$

In  $C$  :

$$Dx = 0.05 \text{ mm} ; Dy = 3.75 \text{ mm} ; Dz = -2.50 \text{ mm} ; DRx = 0.0 \text{ rd} ; DRY = 0.1 \text{ rd} ; DRz = 0.15 \text{ rd}$$

In the total reference, one finds at the points  $B$  and  $C$  :

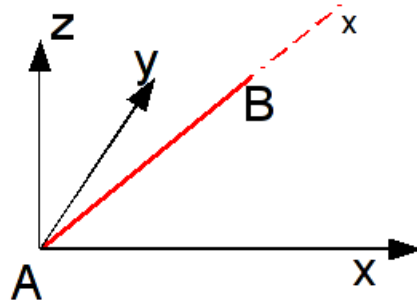
$$\begin{aligned} DX(B) &= \frac{\sqrt{3}}{30} + 5 \frac{\sqrt{3}}{6} (-3\sqrt{6} + 2\sqrt{2}) [mm] & DX(C) &= \frac{\sqrt{3}}{60} + 5 \frac{\sqrt{3}}{24} (-3\sqrt{6} + 2\sqrt{2}) [mm] \\ DY(B) &= \frac{\sqrt{3}}{30} + 5 \frac{\sqrt{3}}{6} (3\sqrt{6} + 2\sqrt{2}) [mm] & DY(C) &= \frac{\sqrt{3}}{60} + 5 \frac{\sqrt{3}}{24} (3\sqrt{6} + 2\sqrt{2}) [mm] \\ DZ(B) &= \frac{\sqrt{3}}{30} + 5 \frac{\sqrt{3}}{6} (-4\sqrt{2}) [mm] & DZ(C) &= \frac{\sqrt{3}}{60} + 5 \frac{\sqrt{3}}{24} (-4\sqrt{2}) [mm] \\ DRX(B) &= \frac{1}{20} (-\sqrt{6} - 2\sqrt{2}) [rd] & DRX(C) &= \frac{1}{40} (-\sqrt{6} - 2\sqrt{2}) [rd] \\ DRY(B) &= \frac{1}{20} (-\sqrt{6} + 2\sqrt{2}) [rd] & DRY(C) &= \frac{1}{40} (-\sqrt{6} + 2\sqrt{2}) [rd] \\ DRZ(B) &= \frac{1}{20} (2\sqrt{6}) [rd] & DRZ(C) &= \frac{1}{40} (2\sqrt{6}) [rd] \end{aligned}$$

### 2.3 Uncertainty on the solution

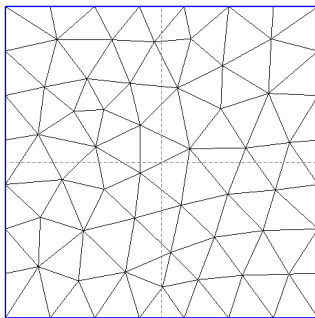
the solution is exact for the theory of the beams of Eulerian (or of Timoshenko because there are no shears). Torsion not intervening, the solution is also valid for elements `POU_D_TG`.

## 3 Modelization A

### 3.1 Characteristic of the modelization



- The segment  $AB$  is cut out in 10 of the same elements length (10.). (Only one element would be sufficient).
- 4 identical calculations are successively done on this mesh with 4 different modelizations:
  - with 10 elements `POU_D_E`
  - with 10 elements `POU_D_T`
  - with 10 elements `POU_D_TG`
  - with 10 elements `POU_D_EM`



Mesh of the section:

- 77 nodes
- 124 `TRIA3`

### 3.2 Characteristic of the mesh

Many nodes: 11

Number of meshes and types: 10 `SEG2`

## 3.3 Quantities tested and results

Modelization	Identification	Reference	% difference	
POU_D_E	BDX	- 6.4664E+00	< 1.0E-9	
	DY	1.4747E+01	< 1.0E-9	
	DZ	- 8.1072E+00	< 1.0E-9	
	DRX	- 2.6390E-01	< 1.0E-9	
	DRY	1.8947E-02	< 1.0E-9	
	DRZ	2.4495E-01	< 1.0E-9	
	CDX	- 1.6022E+00	< 1.0E-9	
	DY	3.7011E+00	< 1.0E-9	
	DZ	- 2.0124E+00	< 1.0E-9	
	DRX	- 1.3195E-01	< 1.0E-9	
	DRY	9.4734E-03	< 1.0E-9	
	DRZ	1.2247E-01	< 1.0E-9	
	POU_D_T	BDX	- 6.4664E+00	< 1.0E-9
		DY	1.4747E+01	< 1.0E-9
DZ		- 8.1072E+00	< 1.0E-9	
DRX		- 2.6390E-01	< 1.0E-9	
DRY		1.8947E-02	< 1.0E-9	
DRZ		2.4495E-01	< 1.0E-9	
CDX		- 1.6022E+00	< 1.0E-9	
DY		3.7011E+00	< 1.0E-9	
DZ		- 2.0124E+00	< 1.0E-9	
DRX		- 1.3195E-01	< 1.0E-9	
DRY		9.4734E-03	< 1.0E-9	
DRZ		1.2247E-01	< 1.0E-9	
POU_D_TG		BDX	- 6.4664E+00	< 1.0E-9
		DY	1.4747E+01	< 1.0E-9
	DZ	- 8.1072E+00	< 1.0E-9	
	DRX	- 2.6390E-01	< 1.0E-9	
	DRY	1.8947E-02	< 1.0E-9	
	DRZ	2.4495E-01	< 1.0E-9	
	CDX	- 1.6022E+00	< 1.0E-9	
	DY	3.7011E+00	< 1.0E-9	
	DZ	- 2.0124E+00	< 1.0E-9	
	DRX	- 1.3195E-01	< 1.0E-9	
	DRY	9.4734E-03	< 1.0E-9	
	DRZ	1.2247E-01	< 1.0E-9	
	POU_D_EM	BDX	- 6.4664E+00	< 1.0E-6
		DY	1.4747E+01	< 1.0E-5
DZ		- 8.1072E+00	< 1.0E-5	
DRX		- 2.6390E-01	< 1.0E-5	
DRY		1.8947E-02	< 1.0E-4	
DRZ		2.4495E-01	< 1.0E-5	
CDX		- 1.6022E+00	< 1.0E-6	
DY		3.7011E+00	< 1.0E-5	
DZ		- 2.0124E+00	< 1.0E-5	
DRX		- 1.3195E-01	< 1.0E-5	
DRY		9.4734E-03	< 1.0E-4	
DRZ		1.2247E-01	< 1.0E-5	

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## 4 Summary of the results

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As one could expect it, the results are very precise. They validate the good taking into account of the predeformations in the beam elements.

The test does not relate to curved beams (POU\_C\_T) because one does not have reference solution.