

Titre : SDLV131 - Simulation d'une jauge de déformation pa[...] Responsable : Harinaivo ANDRIAMBOLOLONA Version

default

# SDLV131 - Simulation of a strain gauge by the command OBSERVATION

#### Summarized:

This test validates the operation of the computation of the mean value of a strain field on an entity given by the user. The strain field thus is estimated that a strain gauge would have measured. One carries out this computation via macro-command OBSERVATION. The treated case is a beam in simple tension modelled by voluminal elements.

This case test also validates the observation of mixed fields: only one call to command OBSERVATION for the statement of fields of different nature (DEPL, QUICKLY,...).

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# **1 Problem of reference**

### 1.1 Geometry



### 1.2 Properties of the material

Young's modulus:  $E = 2.110^{11} N/m^2$ Poisson's ratio: v = 0.3Density:  $\rho = 7800. kg/m^3$ 

### 1.3 Boundary conditions and loadings

One imposes a horizontal displacement  $u_x = 0.0$  on the face surfl.

One imposes a displacement  $u_y=0.0$  on the nodes which are on line central upper face and the line central one of the lower face.

One imposes a displacement  $u_z = 0.0$  on the nodes which are on the central lines of the side sides.

One applies a surface force  $F_B$  to the face surf2 according to the direction x

 $F_{B} = 1000. N/m^{2}$ ,

These boundary conditions allow to obtain a behavior of the beam in simple tension.

### 1.4 Initial conditions

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Without Reference solution

### 2 object

### 2.1 Method of calculating

the strain is estimated from the relative lengthening of the beam.

The lengthening of a beam length L following a longitudinal force F is written:  $\Delta L = \frac{FL}{ES}$ 

In our case, one applies a force per unit of area  $F_B$  at the loose lead of the beam, thus the relative lengthening of the beam puts oneself in the following form:

$$\frac{\Delta L}{L} = \epsilon_{xx} = \frac{F_B}{E}$$

For this case test, one computation results resulting from a static computation, a harmonic computation, a transient computation and of a modal computation.

For the static case, one obtains:

$$\epsilon_{xx} = \frac{F_B}{E}$$
 and  $\epsilon_{yy} = \epsilon_{zz} = -\nu \epsilon_{xx}$ 

For the dynamic cases, the system is governed by the following equation:

$$M \frac{\partial^2 u}{\partial t^2} + K u = F_{ext}$$

For a beam in traction and compression, if one considers a model which contains one element, the mass matrixes M and of stiffness K put oneself in the following form:

$$M = \frac{\rho S L}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \qquad K = \frac{ES}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \qquad \text{with:} \quad u = \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

 $u_1$  and  $u_2$  are displacements of the nodes of the element.

In harmonic response of pulsation  $\,\omega\,$  , displacements of the nodes of the element are governed by the following relation:

$$\frac{ES}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} - \omega^2 \frac{\rho S L}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = \begin{bmatrix} F_1 \\ F_2 \end{bmatrix}$$

By exploiting the second-row forward of this relation, and by applying the boundary conditions (  $u_1=0$  and  $F_2=F=F_BS$  ), one obtains:

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$$u_2 = \frac{F_B}{\frac{E}{L} - \omega^2 \frac{\rho L}{3}} = \Delta L$$

The strain at the loose lead of the beam is written:

$$\varepsilon_{xx} = \frac{\Delta L}{L} = \frac{F_B}{E - \omega^2 \frac{\rho L^2}{3}}$$
 and  $\varepsilon_{yy} = \varepsilon_{zz} = -\nu \varepsilon_{xx}$ 

the reference solution for the transitory solution can be obtained same way. If one applies a longitudinal force  $F(t) = Ft = F_B St$  with an initial condition of beam in equilibrium (initial displacement no one and initial velocity null), one obtains:

$$u_{2}(t) = \frac{3F_{B}}{\rho L \omega_{0}^{2}} \left[ t - \frac{\sin(\omega_{0}t)}{\omega_{0}} \right] = \Delta L \quad \text{with:} \quad \omega_{0}^{2} = \frac{3E}{\rho L^{2}}$$

And the strain at the loose lead of the beam is written:

$$\varepsilon_{xx} = \frac{\Delta L}{L} = \frac{3F_B}{\rho L^2 \omega_0^2} \left[ t - \frac{\sin(\omega_0 t)}{\omega_0} \right] \text{ and } \varepsilon_{yy} = \varepsilon_{zz} = -\nu \varepsilon_{xx}$$

In the case of modal computation, one carries out a test of NON-regression on the strains calculated at the point medium of the beam.

One also simulates a rotation of 90 degrees in order to check the change of reference in OBSERVATION.

#### 2.2 Quantities and results of reference

One tests the value of the average strain on surfaces: surf3 surf4, surf5 and surf6. The got results are then projected to "measure" which understands only the nodes P3 P4, P5 and P6 are the model associated with surfaces surf3 surf4, surf5 and surf6.

For the validation of the static solution, one has chooses:  $F_{B} = 1000. N/m^{2}$ 

For the validation of the harmonic solution, one chose:  $F_B = 1000.(1+2j)N/m^2$  and:  $\omega = 2\pi 200 rd s^{-1}$ 

For the validation of the transitory solution, one chose:  $F_B = 1000 \cdot t N / m^2$  and the solution is tested at time t = 1 s

One also tests the values of the fields obtained by mixed observation.

#### 2.3 Uncertainties on the analytical

solution Solution for the static case, the harmonic case and the transitory case.

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One suggests a solution of NON-regression in the case of the modal strain.

#### 2.4 Bibliographical reference

[R3.08.01] "exact" Elements of beams (right and curved).

### 3 Modelization A

### 3.1 Characteristic of the modelization

One calculates the average strain resulting from a static computation of response with  ${\tt MECA\_STATIQUE}.$ 

One also calls upon operator OBSERVATION for the statements of fields of displacement and strain.

#### 3.2 Characteristics of the mesh

Nodes: 1029 Meshes: 720 HEXA8

#### 3.3 Quantities tested and results

One tests the value of the nodal deformation who are in the middle of the beam.

Identification		Reference	Aster	Tolerance
$\varepsilon$ in P3		4.7619047619	4.7619047619	0.1%
		048D-09	047D-09	
$\varepsilon_{}$ in P3		-1.428571428	-1.428571428	0.1%
		5714D-09	5714D-09	
$\varepsilon_{m}$ in P4 (after rotation of	90°)	4.7619047619	4.7619047619	0.1%
	,	048D-09	047D-09	
$\epsilon_{m}$ in P4 (after rotation of	90°)	-1.428571428	-1.428571428	0.1%
		5714D-09	5714D-09	

Tests for the mixed observation:

Identification	Reference	Aster	Tolerance
$\varepsilon_{xx}$ in P3	4.7619047619 048D-09	4.7619047619 047D-09	0.1%
DX in $P5$ (m)	4.7619047619 048D-09	4.5238095238 095D-09	6.0%

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### 4 Modelization B

# 4.1 Characteristic of the modelization

One calculates the average strain resulting from a harmonic computation. One chooses an excitation frequency equalizes with  $200\,Hz$ .

One also calls upon operator OBSERVATION for the statements of fields of displacement and strain.

### 4.2 Characteristics of the mesh

Nodes: 1029 Meshes: 720 HEXA8

### 4.3 Quantities tested and results

One tests the value of the nodal deformation who are at the loose lead of the beam.

Identification	Reference	Aster	Tolerance
$\varepsilon$ in P5	4.8568623280	4.7813760355	2.0%
	004D-09 +	714D-09 +	
	i9.713724656	i9.562752071	
	0009D-09	1429D-09	
$\epsilon$ in $P5$	-1.457058698	-1.433930338	2.0%
	4001D-09 -	0850D-09 -	
	i2.914117396	i2.867860676	
	8003D-09	1701D-09	
$\epsilon$ in <i>P6</i> (after rotation of 90°	<b>4.</b> 8568623280	4.7792223250	2.0%
	, 004D-09 +	189D-09 +	
	i9.713724656	i9.558444650	
	0009D-09	0377D-09	
$\epsilon$ in <i>P6</i> (after rotation of 90°	-1.457058698	-1.433911032	2.0%
	, 4001D-09 -	9539D-09 -	
	i2.914117396	i2.867822065	
	8003D-09	9079D-09	

Tests for the mixed observation:

Identification	Reference	Aster	Tolerance
$\varepsilon_{xx}$ in P5	4.8568623280 004E-09 + i9.713724656 0009E-09	4.7792696544 452E-09 + i9.558539308 8903E-09	2.0%
Displacement $DX$ in $P5$ (m)	4.8568623280 004E-09 + i9.713724656 0009E-09	4.6188418141 639E-09 + i9.237683628 3278E-09	5.0%

Code_Aster		Version default	
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Vitesseformule <i>DX</i> formulates <i>P5</i> m/s) -1.2206626407315E-05	of it + i6.103313203 6573E-06	-1.160841560 9177E-05 + i5.804207804 5883E-06	5.0%

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### 5 Modelization C

### 5.1 Characteristic of the modelization

One calculates the average strain resulting from a transient computation on physical base. For the resolution of the system, one chooses a temporal discretization equalizes with 0.1 s.

One also calls upon operator OBSERVATION for the statements of fields of displacement and strain.

#### 5.2 Characteristics of the mesh

Nodes: 1029 Meshes: 720 HEXA8

### 5.3 Quantities tested and results

One tests the value of the nodal deformation who are at the loose lead of the beam at time t=1 s.

Identification	Reference	Aster	Tolerance
$\varepsilon_{xx}$ in P5	4.7614812131 879D-09	4.7619095908 375D-09	2.0%
$\epsilon_{zz}$ in $P5$	-1.428444363 9564D-09	-1.428572758 7481D-09	2.0%
$\varepsilon_{_{yy}}$ in $P6$ (after rotation of $90^\circ$ )	4.7614812131 879D-09	4.7619090586 422D-09	2.0%
$\varepsilon_{_{xx}}$ in $P6$ (after rotation of $90^{\circ}$ )	-1.428444363 9564D-09	-1.428572756 4358D-09	2.0%

Tests for the mixed observation:

Identification	Reference	Aster	Tolerance
$\varepsilon_{xx}$ in P5	4.7614812131 879E-09	4.7619090690 47E-09	2.0%
Déplacementformule $DX$ formulates $P5$ (m)	4.7614812131 879E-09	4.5238330203 209E-09	6.0%

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### 6 Modelization D

### 6.1 Characteristic of the modelization

One calculates the average strain resulting from a modal computation with <code>MODE\_ITER\_SIMULT</code>.

One also calls upon operator OBSERVATION for the statements of fields of displacement and strain.

#### 6.2 Characteristics of the mesh

Nodes: 1029 Meshes: 720 HEXA8

### 6.3 Quantities tested and results

One tests the mean value of the strain of the first longitudinal mode of the beam to the nodes which are in the medium and the loose lead. The values of reference are those obtained with the version 10.1 (test of NON-regression). They are given with four significant figures.

Identification		Reference	Aster	Tolerance
$\epsilon_{xx}$ in P3		1.012	1.0120571181 683D+00	0.1%
$\epsilon_{zz}$ in $P3$		-3.063D-1	-3.063127515 8658D-01	0.1%
$\epsilon_{\scriptscriptstyle xx}$ in $P5$		1.762D-1	1.7617357057 031D-01	0.1%
$\epsilon_{zz}$ in $P5$		-4.646D-2	-4.645882565 3459D-02	0.1%
$\epsilon_{yy}$ in $P4$ (after rotation of	90°)	1.017	1.0168194189 613D+00	0.1%
$\epsilon_{\scriptscriptstyle xx}$ in $P4$ (after rotation of	90°)	-3.118D-1	-3.118116138 8859D-01	0.1%
$\epsilon_{yy}$ in $P6$ (after rotation of	90°)	1.533D-1	1.5327248667 577D-01	0.1%
$\epsilon_{\scriptscriptstyle XX}$ in $P6$ (after rotation of	90°)	-4.178D-2	-4.178288109 1537D-02	0.1%

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Tests for the mixed observation:

The purpose of identification	Reference	Aster	% difference
$\varepsilon_{xx}$ in P3	1.01	1.0144135640 865	0.1%
Déplacementformule $DX$ formulates $P3$ (m)	0.7570	0.7570567975 9059	0.1%

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# 7 Summary of the results

These tests are checking the good course of computation of the mean value of the strain field using macrocommand OBSERVATION. The reference solution is analytical for the static response, the harmonic response and the transient response.

This case test also validates the mixed observation of field (DEPL, QUICKLY,...) with only one call to operator OBSERVATION.

The differences between the solutions obtained with Aster and the analytical solutions are very weak. For the results resulting from a modal computation, a test of NON-regression is proposed.