

## SDLS115 – Comparison with the analytical solution of a plate in tension

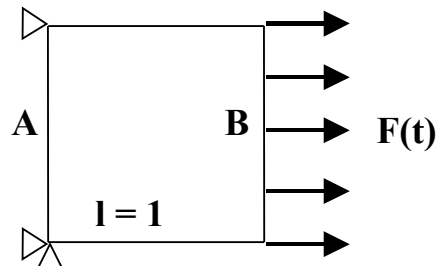
---

### Summarized:

This test validates by the operator the basic operation of `modelization` DKT for a transient computation with an explicit diagram of numerical integration `DYNA_NON_LINE`. The plate is subjected to the boundary conditions corresponding to a simple tension, making it possible to find the analytically calculated response.

## 1 Problem of reference

### 1.1 Geometry



Plates square :  
Length:  $l=1.0\text{ m}$   
Thickness:  $e=0.1\text{ m}$

### 1.2 Properties of the material

Young's modulus,  $E=4.388\ 10^{10}\text{ N/m}^2$

Poisson's ratio,  $\nu=0.0$

Density,  $\rho=2500.0\text{ kg/m}^3$

### 1.3 Boundary conditions and loadings

On with dimensions one A horizontal displacement is imposed  $u_x=0.0$  .

One B applies the linear force to with dimensions one in the direction  $x$  , which depends on time like

$$F(t)=Q_0 E K e \cos(Kl)\sin(\omega t) ,$$

where the following parameters are used:

- $Q_0$  (  $=10^{-4}\text{ m}$  ) - amplitude of the loading
- $E$  - Young's modulus defined above (in  $\text{N/m}^2$  )
- $e$  - the thickness defined above (in  $\text{m}$  )
- $l$  - the dimension of the plate defined above (in  $\text{m}$  )
- $K$  (  $=\frac{\pi}{8l}$  ) the wave number of the analytical solution (in  $\text{m}^{-1}$  )
- $\omega$  - frequency (time  $2\pi$  ), dependant on the wave number  $K$   $K=\omega/c$  ,  $c$

being the celerity of the waves in structure,  $c=\sqrt{\frac{E}{\rho}}$

the introduced parameter setting makes it possible to apply the loading right to obtain the analytical, simply given solution by the parameters  $Q_0$  and  $K$  , and then by other parameters of dimensions and properties structural material.

## 1.4 Initial conditions

At the beginning displacements are worth zero everywhere and the velocities obey the following spatial distribution,

$$v_0(x, y) = \omega Q_0 \sin(K.x)$$

## 2 Reference solution

---

### 2.1 Method of calculating

One deals here with a problem of structure (quasi) - unidimensional subjected to an edge force  $F(t)$ , where the analytical solution can be written like,

$$u(x, t) = Q_0 \cos(Kx) \sin(\omega t)$$

In order to obtain this solution one must apply the force and the initial conditions specified above. The parameters are also commented on there.

### 2.2 Quantities and results of reference

It is displacement  $x$  to the node  $N2$  and the time  $t_{max} = 0.0012 s$ , which must be equal to

$$u(x_{N2}, t) = Q_0 \cos(Kx_{N2}) \sin(\omega t_{max})$$

the value being calculated in the data file starting from the selected values of the parameters.

### 2.3 Uncertainties on the solution

exact Solution.

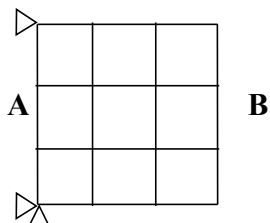
### 2.4 Bibliographical references

S. Timoshenko, *Theory of vibrations*, 1939

## 3 Modelization A

---

### 3.1 Characteristic of the modelization



Modelization: **DKT**

Boundary conditions:

**A** – edge embedded

**B** – linear force

### 3.2 Characteristics of the mesh

Nodes: 16

Meshes: 9 QUAD4

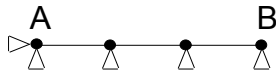
### 3.3 Quantities tested and results

Identification	Reference	Aster	% difference
Displacement $DX$ in $N2$	3.51957D-05	3.51967D-05	0.003

## 4 Modelization B

---

### 4.1 Characteristic of the modelization



Modelization: **BAR**

Boundary conditions:

**A** – node embedded

**B** – force

### 4.2 Characteristics of the mesh

Nodes: 4

Meshes: 3 SEG2

### 4.3 Quantities tested and results

Identification	Reference	Aster	% difference
DX Displacement out of N2	3.51957D-05	3.51967D-05	1.11D-02%
Kinetic energy of the third mesh	9.10387D-02	the purpose of 0.09103866912916 9	3.39D-05%

## 5 Summary of the results

---

This test are principal to check if the combination of modelization `DKT` and operator `DYNA_NON_LINE` functions correctly. The difference between the solution Aster and that of the reference is very weak.