

SDLD321 - Response transient dynamics of a harmonic oscillator with variable damping

Abstract:

The system considered is a harmonic oscillator with 1 degree of freedom under harmonic excitation with resonance. Various depreciation will be considered:

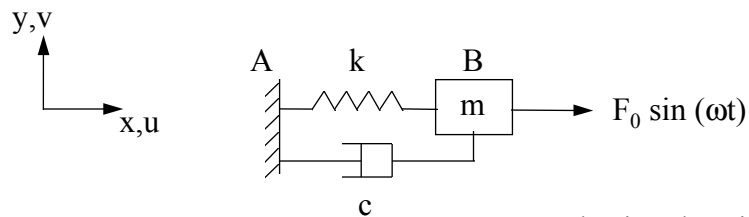
- critical damping,
- average damping,
- very weak damping.

Via this problem, one tests various algorithms DYNA_TRAN_MODAL of the command [U4.54.03] and their capacities with dealing with problems with extreme damping. The results are compared with the exact analytical solutions.

1 Problem of reference

1.1 Geometry

the system is composed of a mass, a spring and a damper. He admits a single degree of freedom in translation.



ω : pulsation d'excitation correspondant
à la résonance du système non amorti

$$\omega = \sqrt{\frac{k}{m}}$$

1.2 Material properties

Stiffness of connection: $k = 25 \cdot 10^3 \text{ N.m}^{-1}$

Point mass: $m = 10 \text{ kg}$

Viscous damping:

$$c = c_{\text{critique}} ; c = 0,01 c_{\text{critique}} ; c = 10^{-5} c_{\text{critique}}$$

with $c_{\text{critique}} = 1000 \text{ kg.s}^{-1}$

1.3 Boundary conditions and loadings

End A clamped.

Force harmonic according to X with the resonance frequency with point: B

$$F(t) = F_0 \sin(\omega t) \text{ for } t \geq 0 \text{ with } F_0 = 5 \text{ N and } \omega = \sqrt{\frac{k}{m}} = 50 \text{ rad.s}^{-1}.$$

1.4 Initial conditions

the system is at rest with $t = 0$: $u(0) = 0$ and $\frac{du}{dt}(0) = 0$.

2 Reference solution

2.1 Method of calculating used for the reference solution

the simple oscillator checks the following equation:

$$m \ddot{u} + c \dot{u} + k u = F_0 \sin(\omega t)$$

$$\text{with } u(0)=0 \text{ and } \dot{u}(0)=0$$

$$\omega : \text{own pulsation of the oscillator } \omega = \sqrt{\frac{k}{m}}$$

The damping criticizes is $c_{critique} = 2m \omega$.

The solution for $c = c_{critique}$ is:

$$u(t) = \frac{F_0}{2k} \left[e^{-\omega t} (1 + \omega t) - \cos(\omega t) \right]$$

The solution for a subcritical damping such as $\frac{c}{c_{critique}} = \xi$ is:

$$u(t) = e^{-\xi \omega t} \left(\frac{F_0}{2 \xi k} \cos(\omega_D t) + \frac{F_0 \omega}{2 k \omega_D} \sin(\omega_D t) \right) - \frac{F_0}{2 \xi k} \cos(\omega t)$$

$$\text{with } \omega_D = \omega \sqrt{1 - \xi^2}$$

2.2 Results reference

Displacement and velocity of the point B .

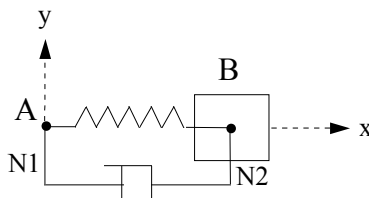
2.3 Uncertainty on the solution

exact analytical Solution.

3 Modelization A

3.1 Characteristic of the modelization

Discrete elements of stiffness, damping and mass.



Characteristics of the elements:

DISCRET :	nodal mass	M_T_D_N
	linear stiffness	K_T_D_L
	straight-line depreciation	A_T_D_L ($c = c_{critique}$)

Boundary conditions: with node $N1$ DDL_IMPO $DX = DY = DZ = 0$.

Names of the nodes: $P_1 = N1$ $P_2 = N2$.

Methods of calculating:

- Integration on modal base with Newmark ($\alpha = 0,25$, $\delta = 0,5$)
Time step $\Delta t = 10^{-3} s$
- Integration on modal base with Eulerian
Time step $\Delta t = 10^{-3} s$

Lasted of observation: $0,5 s$.

3.2 Characteristics of the mesh

Many nodes: 2

Number of meshes and type: 1 mesh SEG2

3.3 Quantities tested and Displacement

- results of the point B

Time (s)	Displacement	Displacement	Tolerance (%)	Displacement	Tolerance (%)
	Reference (m)	NEWMARK Aster (m)		t EULER Aster (m)	
0,06	1,18914 E-4	1,18886 E-4	0.5%	1,18886 E-4	0.5%
0,12	- 9,42819 E-5	- 9,42574 E-5	0.5%	- 9,47822 E-5	0.6%

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0,19	9,97958 E-5	9,97765 E-5	0.5%	9,96206 E-5	0.5%
0,25	- 9,97748 E-5	- 9,97526 E-5	0.5%	- 9,99152 E-5	0.5%
0,31	9,78457 E-5	9,78210 E-5	0.5%	9,83436 E-5	0.6%
0,38	- 9,88705 E-5	- 9,88530 E-5	0.5%	- 9,84730 E-5	0.5%
0,44	9,99961 E-5	9,99754 E-5	0.5%	9,99525 E-5	0.5%

- Velocity of the point B

Time (s)	Velocity Reference ($m.s^{-1}$)	Velocity NEWMARK Aster ($m.s^{-1}$)	Tolerance (%)	Velocity EULER Aster ($m.s^{-1}$)	Tolerance (%)
0,03	3,31400 E-3	3,31363 E-3	0.5%	3,32568 E-3	0.5%
0,09	- 5,13760 E-3	- 5,13729 E-3	0.5%	- 5,13627 E-3	0.65%
0,16	4,93337 E-3	4,93354 E-3	0.5%	4,93088 E-3	0.5%
0,22	- 5,00087 E-3	- 5,00087 E-3	0.5%	- 5,00133 E-3	0.5%
0,28	4,95298 E-3	4,95284 E-3	0.5%	4,95297 E-3	0.5%
0,35	- 4,87813 E-3	- 4,87836 E-3	0.5%	- 4,87801 E-3	0.5%
0,41	4,98415 E-3	4,98423 E-3	0.5%	4,98409 E-3	0.5%
0,47	- 4,99041 E-3	- 4,99035 E-3	0.5%	- 4,99043 E-3	0.5%

3.4 Remarks

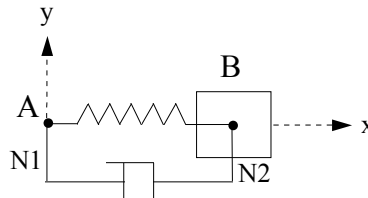
the results are tested on the level of the peaks for the grain of observation selected (10-2s) where the values are most significant.

The mode becomes quasi-permanent after the first period, it is what one must observe by carrying out a transient analysis.

4 Modelization B

4.1 Characteristic of the modelization

Discrete elements of stiffness, damping and mass.



Characteristics of the elements:

DISCRET :	nodal mass	M_T_D_N
	linear stiffness	K_T_D_L
	straight-line depreciation	A_T_D_L ($c = 0,01 c_{critique}$)

Boundary conditions: with node $N1$ DDL_IMPO $DX = DY = DZ = 0$.

Names of the nodes: $P_1 = N1$ $P_2 = N2$.

Methods of calculating:

- Integration on modal base with Fu-Devogelaere
Time step $\Delta t = 10^{-3} s$
- Integration on the basis of modal base with Δt adaptive order 2
Time step initial $\Delta t = 10^{-5} s$
Steps maximum $\Delta t = 10^{-3} s$

Period of observation: $5 s$.

4.2 Characteristics of the mesh

Many nodes: 2

Number of meshes and type: 1 mesh SEG2

4.3 Quantities tested and Displacement

- results of the point *B*

Time (s)	Displacement Reference (m)	Displacement DEVOG Aster (m)	Tolerance (%)	Displacement ADAPT_ORDRE2 Aster (m)	Tolerance (%)
0,06	3,06503 E-4	3,06503 E-4	0.5%	3,06521 E-4	0.5%
0,13	- 5,93807 E-4	- 5,93807 E-4	0.5%	- 5,93729 E-4	0.5%
0,25	- 1,17872 E-3	- 1,17872 E-3	0.5%	- 1,17890 E-3	0.5%
0,69	2,91788 E-3	2,91788 E-3	0.5%	2,91744 E-3	0.5%
1,01	- 3,83901 E-3	- 3,83901 E-3	0.5%	- 3,83567 E-3	0.5%
2,32	6,68206 E-3	6,68206 E-3	0.5%	6,68656 E-3	0.5%
3,64	- 8,19821 E-3	- 8,19821 E-3	0.5%	- 8,204 E-3	0.5%
4,96	9,00847 E-3	9,00847 E-3	0.5%	9,0143 E-3	0.5%

Time (s)	Displacement Reference (m)	Displacement RUNGE_KUTT A_54 Aster (m)	Tolerance (%)	Displacement RUNGE_KUTTA_3 2 Aster (m)	Tolerance (%)
0,06	3,06503 E-4	3.06420E-04	0.5%	3.06443E-04	0.5%
0,13	- 5,93807 E-4	-5.93619E-04	0.5%	-5.93713E-04	0.5%
0,25	- 1,17872 E-3	-1.178373E-3	0.5%	-1.17845E-3	0.5%
0,69	2,91788 E-3	2.91701E-3	0.5%	2.91706E-3	0.5%
1,01	- 3,83901 E-3	-3.83786E-3	0.5%	-3.83772E-3	0.5%
2,32	6,68206 E-3	6.68009E-3	0.5%	6.67939E-3	0.5%
3,64	- 8,19821 E-3	-8.19578E-3	0.5%	-8.19318E-3	0.5%
4,96	9,00847 E-3	9.00579E-3	0.5%	9.00479E-3	0.5%

- Velocity of the point B

Time (s)	Velocity Reference (m.s ⁻¹)	Velocity DEVOG Aster (m.s ⁻¹)	Tolerance (%)	Velocity ADAPT_ORDRE2 Aster (m.s ⁻¹)	Tolerance (%)
0,04	8,95997 E-3	8,95997 E-3	0.5%	8,9722 E-3	0.5%
0,10	- 2,33271 E-2	- 2,33271 E-2	0.5%	- 2,33499 E-2	0.5%
0,22	- 5,20590 E-2	- 5,20590 E-2	0.5%	- 5,2113 E-2	0.5%
0,66	1,40500 E-1	1,40500 E-1	0.5%	1,40591 E-1	0.5%
1,04	1,99889 E-1	1,99889 E-1	0.5%	1,99933 E-1	0.5%
2,36	- 3,39933 E-1	- 3,39933 E-1	0.5%	- 3,39725 E-1	0.5%
3,68	4,10585 E-1	4,10585 E-1	0.5%	4,10008 E-1	0.5%
5,00	- 4,4531 E-1	- 4,45308 E-1	0.5%	- 4,44429 E-1	0.5%

Time (s)	Velocity Reference	Velocity RUNGE_KUTTA_54	Tolerance (%)	Velocity RUNGE_KUTTA_32	Tolerance (%)
	($m.s^{-1}$)	Aster ($m.s^{-1}$)		Aster ($m.s^{-1}$)	
0,04	8,95997 E-3	8.89561E-3	0.5%	8,95719E-3	0.5%
0,10	- 2,33271 E-2	-2,33194E-2	0.5%	-2,33211E-2	0.5%
0,22	- 5,20590 E-2	-5,20435E-2	0.5%	-5,20573E-2	0.5%
0,66	1,40500 E-1	1,40458E-1	0.5%	1,40475E-1	0.5%
1,04	1,99889 E-1	1,99829E-1	0.5%	1,99809E-1	0.5%
2,36	- 3,39933 E-1	-3,39832E-1	0.5%	-3,39767E-1	0.5%
3,68	4,10585 E-1	4,10463E-1	0.5%	4,10403E-1	0.5%
5,00	- 4,4531 E-1	-4,45308E-1	0.5%	-4,45145E-1	0.5%

4.4 Remarks

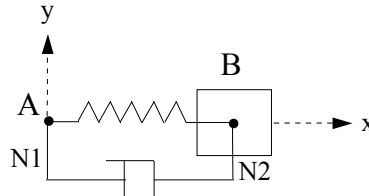
the results is tested on the level of the peaks where the values are most significant.

The period of selected observation makes it possible to see the effect of damping. However, in this interval, the response of the point B remains always transitory but one is close to the permanent mode whose scale of displacement is $10^{-2} m$.

5 Modelization C

5.1 Characteristic of the modelization

Discrete elements of stiffness, damping and mass.



Characteristics of the elements:

DISCRET :	nodal mass	M_T_D_N
	linear stiffness	K_T_D_L
	straight-line depreciation	A_T_D_L ($c = 10^{-5} c_{critique}$)

Boundary conditions: with node $N1$ DDL_IMPO DX = DY = DZ = 0.

Names of the nodes: $P_1 = N1$ $P_2 = N2$.

Methods of calculating:

- Integration on modal base with Newmark ($\alpha = 0,25$, $\delta = 0,5$)
Time step $\Delta t = 10^{-3}s$
- Integration on modal base with Eulerian
Time step $\Delta t = 10^{-3}s$

Lasted of observation: 5 s .

5.2 Characteristics of the mesh

Many nodes: 2

Number of meshes and type: 1 mesh SEG2

5.3 Quantities tested and Displacement

- results of the point B

Time (s)	Displacement	Displacement	Tolerance (%)	Displacement	Tolerance (%)
	Reference (m)	NEWMARK Aster (m)		EULER Aster (m)	
0,06	3,11105 E-4	3,10936 E-4	0.5%	3,11181 E-4	0.5%
0,13	- 6,13250 E-4	- 6,13016 E-4	0.5%	- 6,13380 E-4	0.5%

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0,25	- 1,25380 E-3	- 1,25304 E-3	0.5%	- 1,25418 E-3	0.5%
0,69	3,44945 E-3	3,44691 E-3	0.5%	3,45069 E-3	0.5%
1,01	- 4,88729 E-3	- 4,89081 E-3	0.5%	- 4,88547 E-3	0.5%
2,32	1,12876 E-2	1,12475 E-2	0.5%	1,13069 E-2	0.5%
3,64	- 1,77960 E-2	- 1,77100 E-2	0.5%	- 1,78360 E-2	0.5%
4,96	2,43613 E-2	2,42198 E-2	0.5%	2,44242 E-2	0.5%

- Velocity of the point B

Time (s)	Velocity Reference ($m.s^{-1}$)	Velocity NEWMARK Aster ($m.s^{-1}$)	Tolerance (%)	Velocity EULER Aster ($m.s^{-1}$)	Tolerance (%)
0,04	9,09284 E-3	9,08897 E-3	0.5%	9,08230 E-3	0.5%
0,10	- 2,39724 E-2	- 2,39637 E-2	0.5%	- 2,40269 E-2	0.5%
0,22	- 5,49964 E-2	- 5,49680 E-2	0.5%	- 5,48752 E-2	0.5%
0,66	1,64958 E-1	1,64879 E-1	0.5%	1,64882 E-1	0.5%
1,04	2,56456 E-1	2,56547 E-1	0.5%	2,57280 E-1	0.5%
2,36	- 5,79010 E-1	- 5,80019 E-1	0.5%	- 5,81033 E-1	0.5%
3,68	8,97631 E-1	9,00729 E-1	0.5%	9,00668 E-1	0.5%
5,00	- 1,21164	- 1,21829	0.5%	- 1,21531	0.5%

5.4 Remarks

the results are tested on the level of the peaks where the values are most significant.

In the interval of observation, one remains very below permanent mode in resonance whose scale of displacement is $10m$.

6 Summary of the results

For the modelization A, the results got as well in displacement as of velocity have an absolute error largely lower than 1 % compared to the analytical solution.

The diagram of integration of Newmark is shown more precise than the diagram of Eulerian.

A 1 % of critical damping (modelization B), the diagram of Fu-Devogelaere integration is of a frightening accuracy (not error compared to the reference solution).

The diagram with time step adaptive of order 2 also gives results to very small percentage of error.

For very weak depreciation (modelization C), one will note a better accuracy for the diagram of integration of the Eulerian type than for a diagram of the Newmark type. For this last, the error increases according to time but remains lower all the same than 1 % .